



Prestons Park Subdivision
GEOTECHNICAL REPORT FOR LOT 931
CDL Land NZ Limited

8 October 2024
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to life*

Document control record

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Contents

1	Executive Summary	1
2	Introduction	2
3	Site Conditions	3
3.1	Site Description	3
3.2	Surface Water	3
3.3	Regional Geology	3
3.4	Site Earthworks	3
3.5	Technical Classification	3
4	Geotechnical Assessment	5
4.1	General	5
4.2	Previous Investigations	5
4.3	Hand Held Testing	5
4.4	Dynamic Cone Penetrometers	5
4.5	Groundwater Levels	5
4.6	Topsoil Depth	5
5	Foundation Recommendations	7
5.1	Foundation Types	7
5.2	Foundation Construction Recommendations	7
6	References	8
7	Explanatory Statement	9

Appendices

Appendix A

Figures

Appendix B

Test Results

1 Executive Summary

CDL Land NZ Ltd has commissioned Aurecon New Zealand Ltd to undertake site specific geotechnical investigations and assessments for individual residential lots for building consent purposes for the Prestons Park Subdivision Stage 2. This report documents the results of the geotechnical assessment and presents geotechnical comments together with foundation recommendations for the residential building on Lot 931 at the Prestons Park Subdivision.

As part of the subdivision development extensive geotechnical testing, including cone penetration tests (CPTs), were carried out to define the liquefaction potential at the overall subdivision. **Based on our information the property is likely to perform to a level equivalent to Technical Category 2 (TC2 - Yellow) set by the Ministry of Business, Innovation and Employment (MBIE) in their guidelines issued in December 2012.**

The site development has included extensive bulk earthworks. All earthworks and compliance testing have been undertaken in accordance with the earthworks specification. The engineering sign off and bulk earthworks are documented in the Geotechnical Completion Report. The earthfill at the subdivision construction stage was placed and signed off in accordance with NZS4431:1989.

The geotechnical site specific assessment comprised a review of previous geotechnical investigations undertaken for the subdivision development and a site specific geotechnical investigation. A single hand auger borehole was undertaken to assess the upper soil profile and groundwater level. Two dynamic cone penetrometers (DCPs) were carried out on the lot, one of the DCPs was located immediately adjacent to the hand auger borehole with the second DCP located on the opposite half of the lot.

Based on our knowledge of the site setting and our past work we recommend that any residential structure constructed on Lot 931 be founded on a TC2 type 'enhanced foundation slab' as per Option 3 or 4 from the MBIE Guidelines (2012) Section 5.3.1.

These foundations require a minimum geotechnical ultimate bearing capacity of at least 200kPa, which for Lot 931 is achieved at a minimum depth of 0.4m below ground level as this represents the current thickness of the topsoil and loose material.

This report shall be read as a whole. Our limitations are presented in Section 7.

2 Introduction

CDL Land NZ Ltd is currently undertaking a large residential subdivision with associated commercial lots. Previously Aurecon New Zealand Ltd has undertaken detailed geotechnical investigations and assessments for the purpose of the plan change, subdivision resource consent application, liquefaction assessment, technical classification of the entire subdivision, and observation of bulk earthworks construction.

Aurecon New Zealand Ltd has since been commissioned to undertake site specific geotechnical investigations and assessment of individual lots for building consent application purposes. The site which this report is focused on comprises a single lot, designated Lot 931 at the Prestons Park Subdivision to the south of Prestons Road in Marshland, Christchurch. This report documents the results of the geotechnical assessment and presents geotechnical comments together with foundation recommendations for the proposed residential building on the lot. We note that at the time of writing this report the location and structural form of the dwelling were unknown but is expected to comprise a timber or steel framed building with lightweight cladding.

This report shall be read as a whole. Our limitations are presented in Section 7.

3 Site Conditions

3.1 Site Description

The Prestons Subdivision is located on the northern fringes of Christchurch City. The site is made up of a series of adjacent properties forming an irregular and elongated rectangle shape, orientated approximately north to south. The total area of the overall Prestons Subdivision site is approximately 190ha. The site can be separated into two distinct blocks, Prestons North which runs from Lower Styx Road in the north through to Prestons Road in the south and the Prestons Park development which continues from Prestons Road through to Mairehau Road to the south.

This building consent report is for Lot 931 which is located in Stage 2 of the Prestons Park Subdivision development (refer Figure 1 in Appendix A). The lot is bordered by residential lots to the north, west and east and Mairehau Road to the south.

3.2 Surface Water

There are no natural sources of surface water on the subdivision. There is a stormwater channel to the north and stormwater basins to the north and northeast of Lot 931.

3.3 Regional Geology

The geology of the site is described in the 1:250,000 scale geological map – ‘*Geology of the Christchurch Area*’, published in 2008 by the Institute of Geological and Nuclear Sciences. Note this map has been referenced as it is at an appropriate scale and covers the entire site. The map indicates the underlying geology comprises ‘*Dominantly sand of fixed and semi-fixed dunes and beach deposits*’ and ‘*drained peat swamp*’. The subdivision geotechnical investigation identified that this area was predominantly underlain by aeolian (dune) and beach sand deposits.

The GNS Active Fault System database (GNS, 2012a) indicates that the site is located approximately:

- 27km northeast of the eastern end of the Greendale Fault System. Movement on the Greendale Fault System was responsible for the Magnitude 7.1 Darfield (Canterbury) Earthquake on 4 September 2010.
- 14km north of the epicentre of the Magnitude 6.2 Christchurch Earthquake on 22 February 2011.
- 12km northwest of the Magnitude 6.0 earthquake on 13 June 2011 (GNS, 2012b).
- 8km northwest of the Magnitude 5.9 earthquake on 23 December 2012.

3.4 Site Earthworks

Site earthworks have been undertaken to prepare the site for residential building purposes. Historical subdivision scale earthworks involved the cutting of an existing sand dune, further cutting to remove unsuitable shallow material and subsequent engineered filling. The site finished surface levels exist in a combination of cut and fill.

All earthworks and compliance testing have been undertaken in accordance with the earthworks specification. Details of the site bulk earthworks are provided in the Aurecon report titled ‘*Prestons Park Subdivision – Stage R3, S3, Z2, T4, T5, and J1 Geotechnical Completion Report Rev 1*’ dated 26 September 2024. The Geotechnical Completion Report has been completed as part of the requirements of NZS4404:2010 ‘*Land development and subdivision infrastructure*’ and Christchurch City Council ‘*Infrastructure Design Standards - Part 4: Geotechnical Requirements*’. The compacted fill was placed and signed off in accordance with NZS4431:1989 ‘*Code of practice for earth fill for residential development*’.

3.5 Technical Classification

The landcheck maps historically provided on The Canterbury Earthquake Recovery Authority (CERA) website indicated the site is classified as rural and unmapped. The technical categories were

developed by the Ministry for Business, Innovation and Employment (MBIE) with the support from the Engineering Advisory Group (EAG) in mid-2011 and hence prior to the start of the subdivision development. The site has since been modified by civil engineering and bulk earthworks.

As part of the subdivision consent geotechnical assessment, site specific liquefaction assessment was carried out over the wider subdivision area. This is detailed in Aurecon's reports titled '*Prestons Road Subdivision, Geotechnical Assessment Report for Resource Consent*', Revision 2 dated 5 March 2012, '*Prestons Road Subdivision, Detailed Geotechnical Design Report*', Revision 2 dated 12 July 2012 and '*Prestons South Subdivision Resource Consent Geotechnical Report*', Revision 1 dated 6 June 2013.

Based on the results from our liquefaction assessment and bulk earthworks undertaken on the wider subdivision, Lot 931 is likely to perform to a level equivalent to Technical Category 2 (TC2 - Yellow) in the MBIE Guidelines issued in December 2012.

4 Geotechnical Assessment

4.1 General

The geotechnical assessment comprised a review of relevant previous geotechnical investigations and construction data, and an in-situ geotechnical investigation. The geotechnical investigation comprised handheld testing on the individual lot. A single hand auger borehole was undertaken to assess the upper soil profile and the groundwater level. Two dynamic cone penetrometers (DCPs) were carried out on the lot. The handheld testing was undertaken on the lot following the site earthworks. The testing density is considered reasonable as the geotechnical engineers were involved in the observation of the civil engineering works.

4.2 Previous Investigations

The previous geotechnical testing and investigations undertaken by Aurecon across the entire subdivision site comprised a combination of cone penetrometer tests (CPT) and machine excavated test pits. Further details regarding these investigation locations are found in the Aurecon report titled '*Prestons Park Subdivision – Stage R3, S3, Z2, T4, T5, and J1 Geotechnical Completion Report Rev 1*' dated 26 September 2024.

4.3 Handheld Testing

One hand auger borehole was undertaken on the lot as part of the geotechnical investigation to assess the upper soil profile and the groundwater level. The test location and the borehole log have been uploaded to the New Zealand Geotechnical Database (NZGD). The borehole log is attached in Appendix B.

The hand auger borehole was terminated when soil samples became too dense to auger. The hand auger borehole indicates ground conditions comprise topsoil underlain by gravel.

The logging of the recovered hand auger samples was undertaken in accordance with the New Zealand Geotechnical Society's "*Guideline for the Field Classification and Description of Soil and Rock for Engineering Purposes: 2005*".

4.4 Dynamic Cone Penetrometers

Two DCPs were carried out with the locations and results uploaded to the NZGD. The DCP test results are attached in Appendix B. One of the DCPs was located adjacent to the hand auger borehole, with the second DCP located on the opposite half of the lot.

DCP logs indicate 2 blows per 100mm penetration, which is equivalent to an ultimate bearing capacity of 200kPa, was consistently encountered from a depth of 0.1m.

4.5 Groundwater Levels

Groundwater was not encountered due to shallow auger refusal. Nearby investigations indicate that groundwater is typically encountered at approximately 1m to 2m depth. We note that the groundwater level could vary depending on the time of year.

A review of the NZGD indicates that regional groundwater levels are likely to be in the order of 1m to 2m below ground level. These groundwater levels are reasonably consistent with those measured during the investigation, when taking into account the subdivision earthworks filling and cutting.

4.6 Topsoil Depth

The depth of topsoil has been determined from the single hand auger and two DCPs undertaken on site. We note that the topsoil depths may vary by +/- 100mm between the testing locations and hence it is recommended that at least a 100mm tolerance should be allowed for building foundation excavations.

If greater certainty of topsoil depths across the whole site is required for construction costing, then further testing would be necessary across the footprint of the proposed structure (unknown at the time of writing this report). **The responsibility and liability for any additional testing shall be met by the section purchaser.**

5 Foundation Recommendations

5.1 Foundation Types

It is recommended that residential structures that are built on Lot 931 are founded on shallow foundations. Due to the potential for liquefaction induced lateral spreading adjacent to Lot 931 we recommend that the residential building is founded on a TC2 type 'enhanced foundation slab' as per Option 3 or 4 from the MBIE Guidelines (2012) Section 5.3.1. The site investigation results indicate that approximately 350mm of topsoil was present, which will need to be removed and the clean subgrade exposed for foundation construction.

Based on our understanding of the site works and the investigation results, we recommend that MBIE Guidelines (2012) Section 5.3.1 compliant foundations are embedded a minimum depth of 0.4m below current ground level.

5.2 Foundation Construction Recommendations

Topsoil should be removed from beneath the floor slab and compacted hardfill placed under the floor slab to the required level. In addition, the following recommendations should be considered:

1. To ensure that excavated footings are adequately founded we recommend that excavations should be undertaken by a digger fitted with a smooth edge bucket. The base should be tidily trimmed by hand, lightly compacted with a plate compactor and immediately covered with a concrete tidy slab or 100mm of compacted granular hardfill. A suitable qualified engineer with foundation experience or territorial authority building inspector should be retained to verify the adequate founding has been achieved and all loose and soft or compressible material was removed.
2. If there are significant soft layers beneath a sub-excavation, the softer material will have to be excavated and backfilled with 10MPa concrete or compacted hardfill underneath all load bearing foundation elements. For the hardfill, the sub-excavation will have to have plan dimensions of $B+2D$ where B is the footing width and D is the depth of undercut. For 10MPa concrete the sub-excavation can have the same dimensions as the footing.
3. Depending on the soil encountered, the time of year, and fluctuations of the groundwater level, it is possible that excavations may encounter groundwater. The building contractor is to take appropriate measures to deal with any groundwater ingress into foundation excavations and keep the excavations and backfill free of groundwater intrusions until the footings are cast.
4. The footing excavations are likely to expose layers of granular soils, which can be adversely affected by rainfall or stormwater. Foundation excavations should not be left open for more than three days. In addition, rainfall and stormwater should not be allowed to pond within the footing excavations as this may affect the bearing capacity of the subsoils.

6 References

Forsyth, Barrell & Jongers, (compilers), 2008. *Geology of the Christchurch Area*. Institute of Geological and Nuclear Sciences, 1:250,000 Geological Map 16.

New Zealand Geotechnical Database (2020). <https://www.nzgd.org.nz>

Christchurch City Council, 2010. *Infrastructure Design Standards - Part 4: Geotechnical Requirements*.

Geonet, 2012. <ftp://ftp.geonet.org.nz/strong/processed/Proc> (23/10/12)

GNS, 2012a. <http://maps.gns.cri.nz/website/af/viewer.htm> (23/10/12)

GNS, 2012b. <http://www.gns.cri.nz/Home/News-and-Events/Media-Releases/earthquake-part-of-aftershock-sequence> (23/10/12)

Ministry of Business Innovation and Employment (MBIE), 2012 '*Repairing and rebuilding houses affected by the Canterbury earthquakes*'.

NZGS, 2005. *Guidelines for the Classification and Field Description of Soils and Rocks in Engineering*. NZ Geotechnical Society Inc, Wellington, New Zealand.

NZS 4404:2010 *Land development and subdivision infrastructure*. Standards New Zealand, Wellington, New Zealand.

NZS 4431:1989 *Code of practice for earth fill for residential development*. Standards New Zealand, Wellington, New Zealand.

7 Explanatory Statement

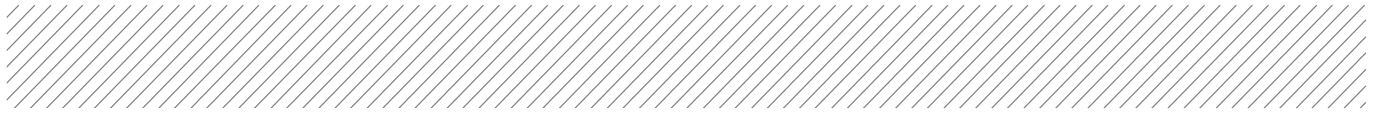
We have prepared this report in accordance with the brief as provided. The contents of the report are for the sole use of the Client for the purpose of building consent application only, and no responsibility or liability will be accepted to any other third party. Data or opinions contained within the report may not be used in other contexts or for any other purposes without our prior review and agreement.

The recommendations in this report are based on data collected at specific locations and by using suitable investigation techniques with limited site coverage. Only a finite amount of information has been collected to meet the specific financial and technical requirements of the Client's brief and this report does not purport to completely describe all the site characteristics and properties. The nature and continuity of the ground and groundwater between test locations has been inferred using experience and judgment and it must be appreciated that actual conditions could vary from the assumed model.

Subsurface conditions relevant to construction works should be assessed by contractors who can make their own interpretation of the factual data provided. They should perform any additional tests as necessary for their own purposes.

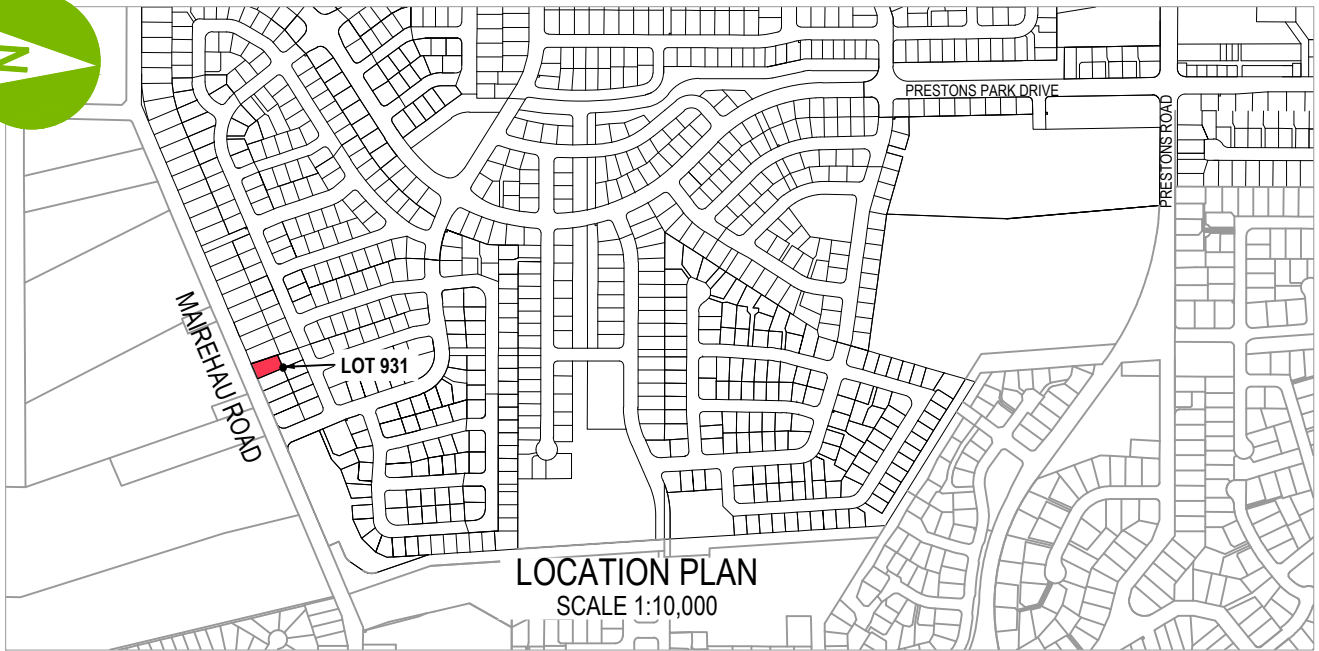
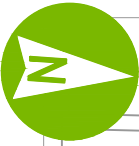
Subsurface conditions, such as groundwater levels, can change over time. This should be borne in mind, particularly if the report is used after a protracted delay.

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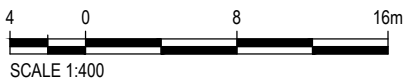



Appendix A

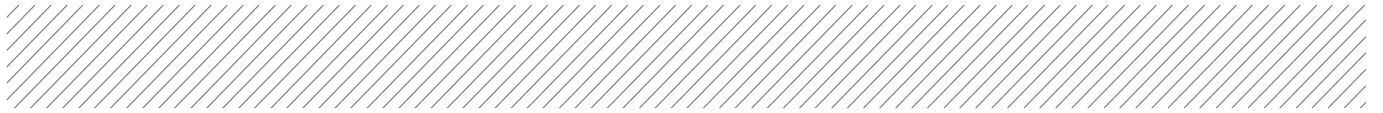
Figures



LOCATION PLAN
SCALE 1:10,000



 <small>www.aurecongroup.com</small>	REV	DATE	REVISION DETAILS	APPROVED	DRAWN	DESIGNED	PROJECT	INFORMATION	
	A	20/09/24	ISSUE FOR INFORMATION	J KUPEC	M KORAKO	C DOHERTY		PRESTONS PARK	PROJECT No.
CLIENT PRESTONS PARK					CHECKED		TITLE	SCALE	SIZE
					J MUIRSON			1:400	A4
					APPROVED			DRAWING No.	REV
					J KUPEC	DATE		20/09/24	GO-PS-S5-IN-931
					J KUPEC		LOT 931 SITE LOCATION PLAN FIGURE 1		



Appendix B

Test Results

DRILLING INFORMATION

Method: Hand Auger
Equipment: Scala Penetrometer
Contractor: N/A

CO-ORDINATES: NZTM

Easting: 1573706m
Northing: 5185352m
Ground Level: 13.30m (CDD)

Date started: 12/09/2024
Date completed: 12/09/2024
Inclination: 90°
Orientation: N/A

Logged by: AP
Input by: AP
Checked by: RL
Verified by: MvL

Depth (m)	Graphic Log	Soil Description	Testing	● Scala Penetrometer Test (Blows/100mm) 3 6 9 12 15 18 21 24 27	Additional Observations	R.L. (m)
		0m: Sandy SILT with trace gravel and rootlets; dark brown. Dry; sand, fine to coarse; gravel, fine to coarse, subrounded to subangular. [TOPSOIL]				13
1		Terminated at 0.35m (Effective refusal)				12
2						11

REMARKS:

[1] Coordinates are approximate (+/- 5m), recorded in the field with a mobile device.
[2] RL is approximate (+/-0.5m) based on design contours.

Water Level Readings:

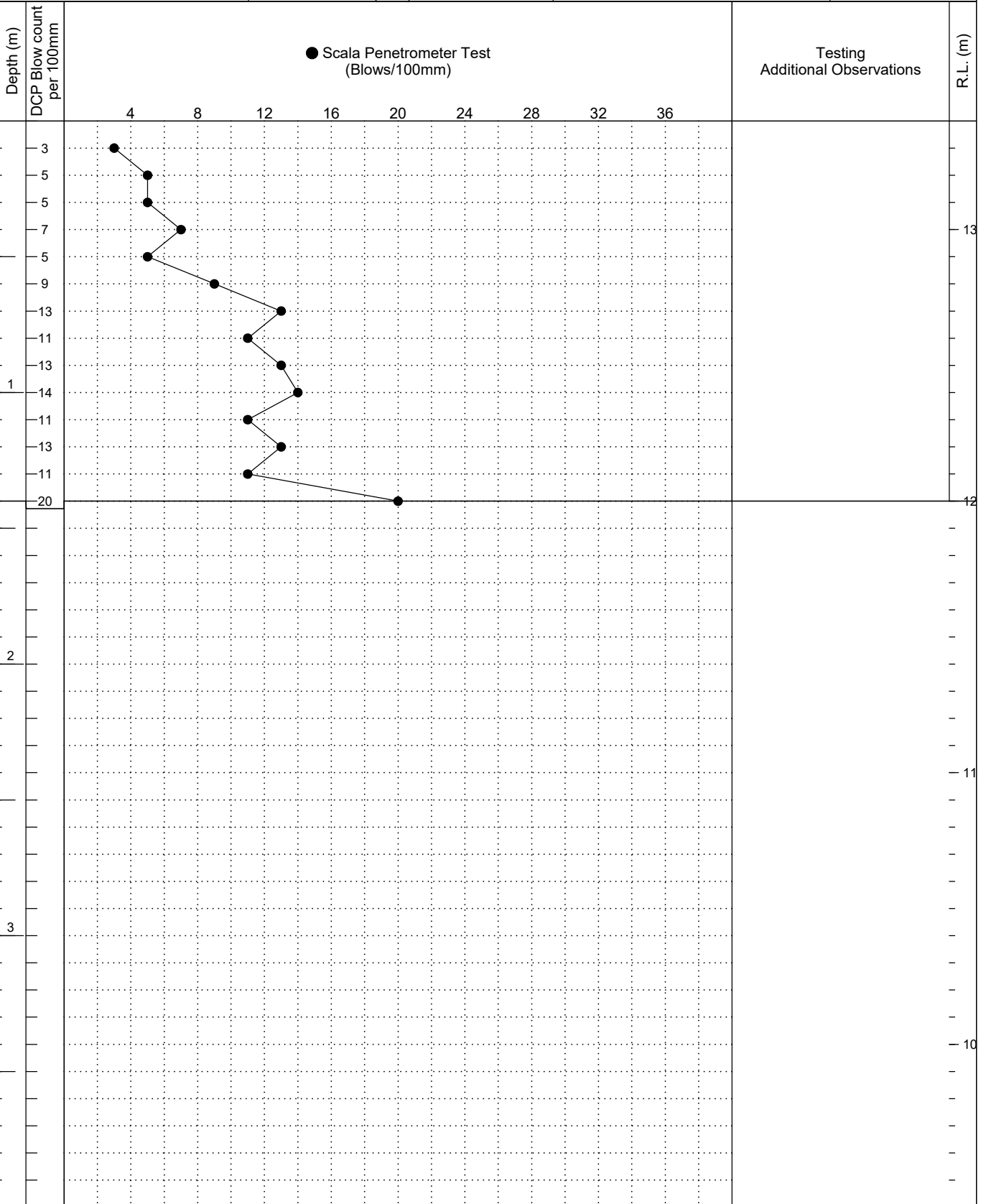
Date Time | Hole Depth | Water Level
No water level recorded

DRILLING INFORMATION
Method: Scala Penetrometer
Equipment: Scala Penetrometer
Contractor: N/A

CO-ORDINATES: NZTM
Easting: 1573697m
Northing: 5185372m
Ground level: 13.40m
(CDD)

Date started: 12/09/2024
Date completed: 12/09/2024
Inclination: 90°
Orientation: N/A

Logged by: AP
Input by: AP
Checked by: RL
Verified by: MVL



REMARKS:
[1] Coordinates are approximate (+/- 5m), recorded in the field with a mobile device.
[2] RL is approximate (+/- 0.5m) based on design contours.

Water Level Readings:
Date Time | Hole Depth | Water Level
No water level recorded



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